

***Revisions to the
Delaware Sediment & Stormwater Regulations***

***Technical Subcommittee Meeting
September 15, 2011***

Proposed Revisions to Delaware Sediment & Stormwater Regulations

Regulatory Language Update

ELGs for Construction & Development Industry



Federal Register

Tuesday,
December 1, 2009

Part III

**Environmental
Protection Agency**

40 CFR Part 450
Effluent Limitations Guidelines and
Standards for the Construction and
Development Point Source Category; Final
Rule

4.0 Construction Site SWM

- Federal Rule (Feb. 1, 2010)
 - Effective on or about August 1, 2011, Numerical Effluent Limit of 280 ntu applies to all construction sites with greater than 20 acres disturbed for all storms less than the 2-YR frequency
 - 20 acre disturbance threshold will roll back to 10 acres effective Feb. 1, 2014

4.0 Construction Site SWM

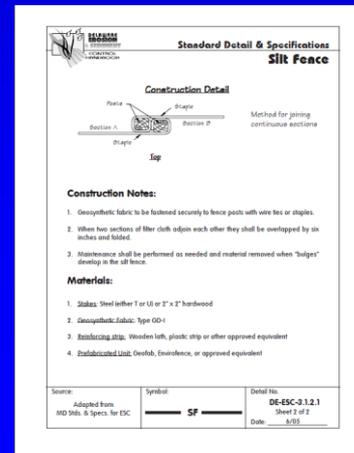
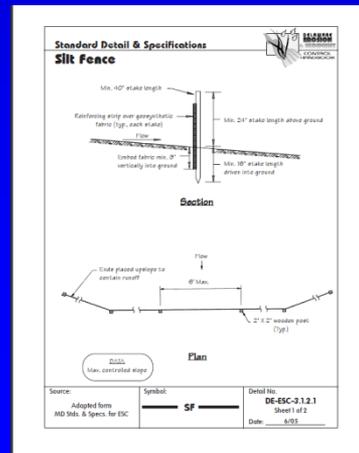
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 - 20 acre disturbance threshold will roll back to 10 acres effective Feb. 2014

4.4 Limits on Land Disturbance

- ≤ 20 acres drainage = Standard Details
- > 20 acres drainage = Engineered Design (2-yr rainfall, “newly graded area” CNs)
- 20 acre max disturbed area to a common discharge point



TMDLs

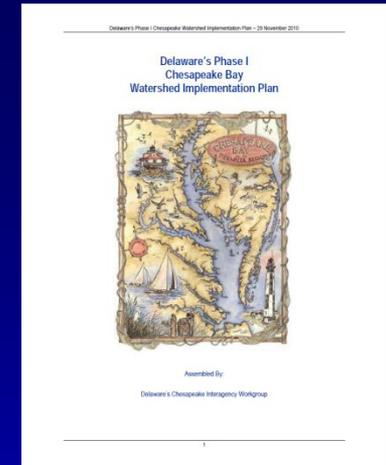


Executive Order 13508
**Draft Strategy for
Protecting and Restoring
the Chesapeake Bay**
November 9, 2009

Developed by the Federal Leadership Committee for the Chesapeake Bay

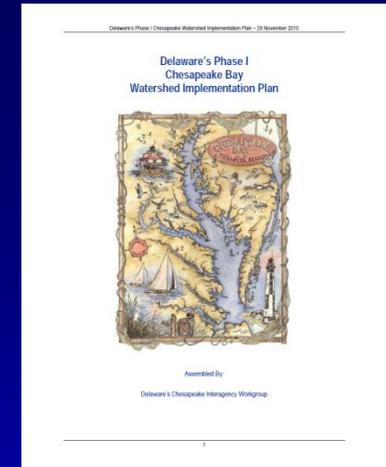
The cover features a stylized illustration of a heron standing on a rock. Below the title, there are seven circular logos representing the member agencies of the Federal Leadership Committee for the Chesapeake Bay: EPA, NOAA, USFWS, USDA, and others.

Chesapeake WIP Delaware Stormwater Phase I High Level Commitments for Urban Sector



- Promulgate new statewide stormwater rules with performance standards for new and redevelopment
- Develop a system for tracking inspections and compliance information
- Implement voluntary programs or regulatory tools to limit turfgrass fertilizer use

Chesapeake WIP Delaware Stormwater Phase I High Level Commitments for Urban Sector



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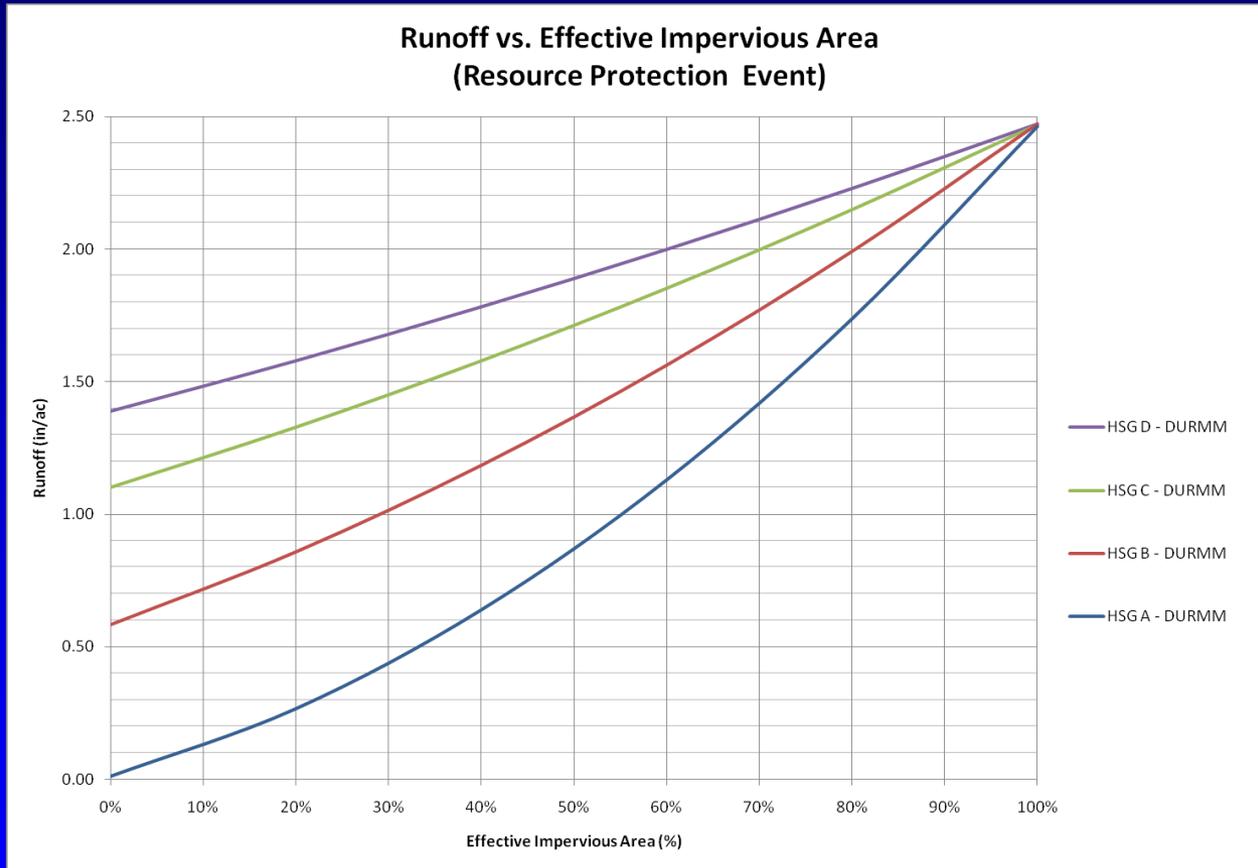
Section 5.0

Performance Criteria for Post-Construction Stormwater Management

5.2 Resource Protection Event Criteria

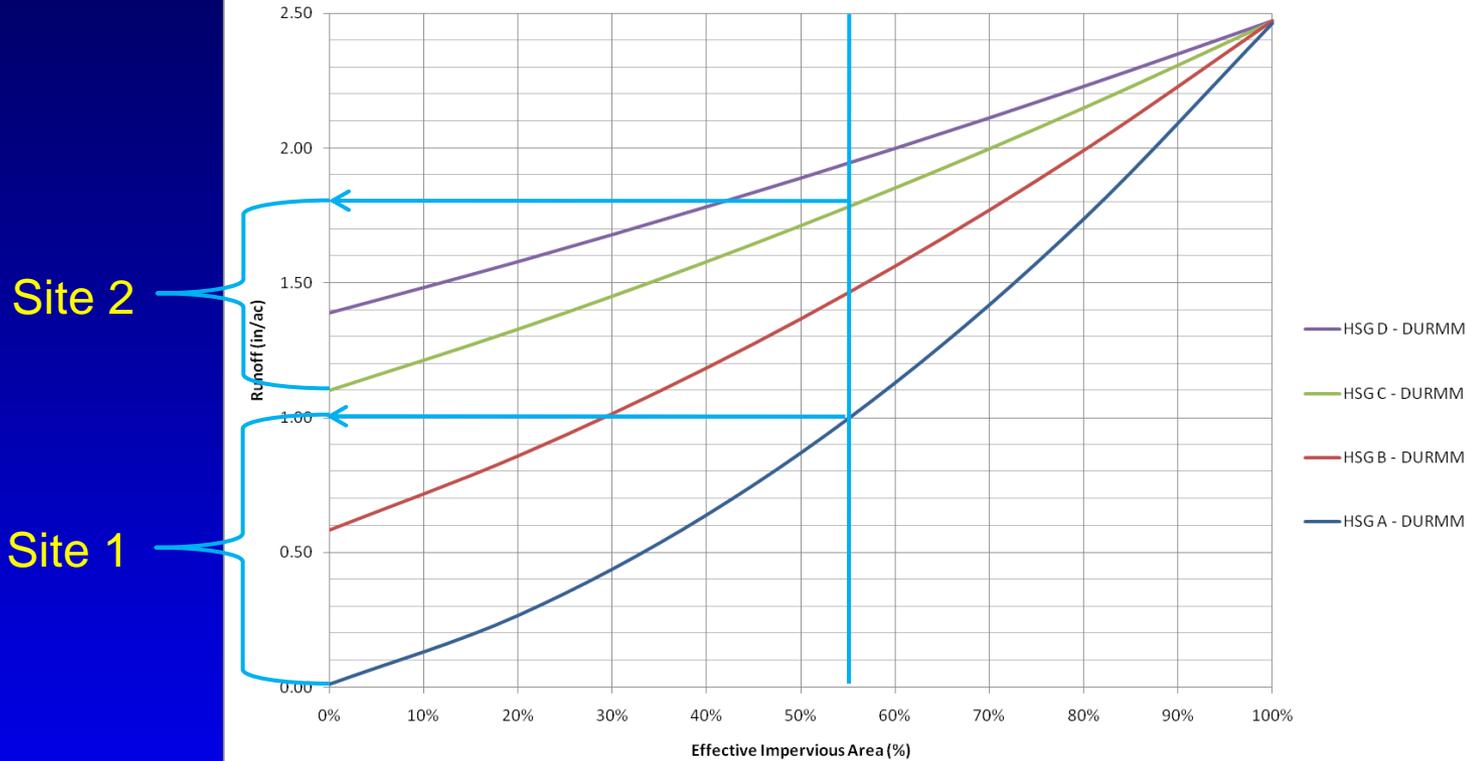
- *Section 5.2.3.1: Runoff from disturbed areas that were wooded or meadow in the pre-developed condition shall be reduced using runoff reduction practices to an equivalent wooded condition.*
- *Section 5.2.3.2: All remaining disturbed areas shall employ runoff reduction practices to achieve an equivalent 0% effective imperviousness.*

Proposed Minimum RR for New Development



Equivalent 0% Effective Imperviousness in LOD

Runoff vs. Effective Impervious Area
(Resource Protection Event)

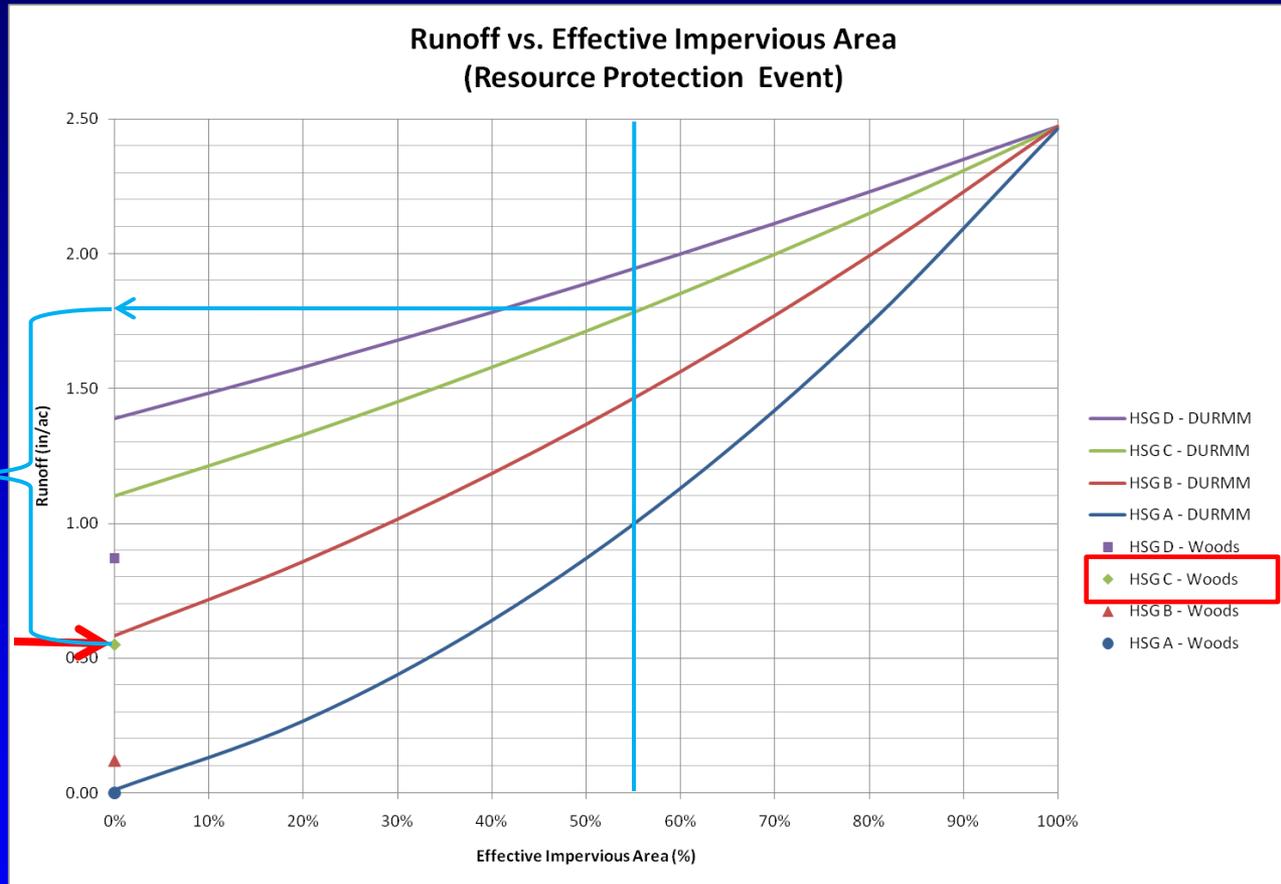


Site 1: 55% Impervious, HSG A Soil
Runoff = 1.0"
Minimum RR = 1.0" – 0" = 1.0" (100% Reduction)

Site 2: 55% Impervious, HSG C Soil
Runoff 1.8"
Minimum RR = 1.8" – 1.1" = 0.7" (38% Reduction)

Existing Woods/Meadow in LOD

Site 2



Site 2: 55% Impervious, HSG C Soil

Runoff 1.8"

Minimum RR = 1.8" - 0.55" = 1.25" (70% Reduction)

5.2 Resource Protection Event Criteria

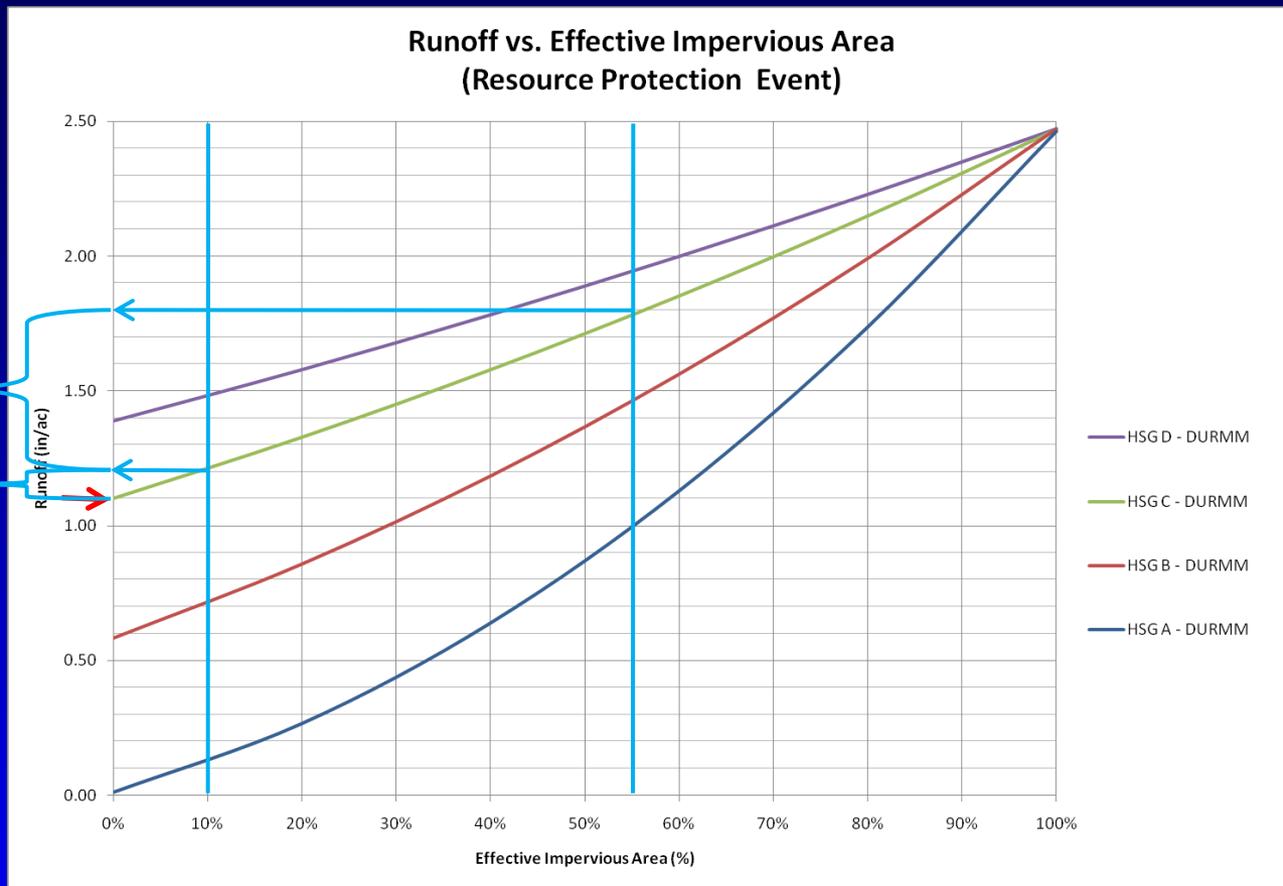
- *5.2.3.2.2 An offset shall be provided for the portion of the RPr that does not meet the minimum runoff reduction requirements.*

Proposed Revisions to Delaware Sediment & Stormwater Regulations: Fee-In-Lieu

- Equivalent to cost to treat runoff volume not managed
- Based on land acquisition, construction, and maintenance costs for bioretention
- Analysis was performed by Center for Watershed Protection using regional data
- Fee = **\$23/cu.ft.** runoff volume

Site 3
RR

Offset



Site 3: 55% Impervious, HSG C Soil

Runoff = 1.8"

Minimum RR = 1.8" – 1.1" = 0.7" (38% Reduction)

Actual RR = 1.8" – 1.2" = 0.6" (33% Reduction)

Offset Volume = 1.2" – 1.1" = 0.1" = 0.1 ac-in/ac = 363 cf/ac

Offset Fee = \$23/cf x 363 cf/ac = \$8,349/ac

5.2 Resource Protection Event Criteria

- *5.2.3.3 Additional water quality treatment BMPs shall be provided if the runoff reduction requirements of this section are not sufficient to meet Total Maximum Daily Load (TMDL) requirements for the receiving water. An offset shall be provided for any portion of the RPs that does not meet TMDL requirements.*

Proposed Revisions to Delaware Sediment & Stormwater Regulations: Fee-In-Lieu Option

- RPs offset fee-in-lieu may be further reduced by implementing additional water quality treatment practices
- Offset fee-in-lieu reduction shall be equivalent to the combined **TN** removal for those practices

Treatment BMP Removal Efficiencies*

4.3.3 Dry Extended Detention Ponds

General Application
Stormwater BMP



Description: A surface storage basin or facility designed to provide water quantity control through detention of stormwater runoff. A dry extended detention pond can be used for water quality treatment purposes as well as for flood control.

KEY CONSIDERATIONS

DESIGN GUIDELINES:

- Maximum contributing drainage area of 75 acres.
- A sediment forebay or equivalent upstream pretreatment must be provided.
- Minimum flow length to width ratio for the pond is 1.5:1. The pond shall be sized to detain the volume of runoff to be treated for a minimum of 24 hours.
- Side slopes to the pond shall not exceed 3:1 (h:v) on one side of the pond to facilitate access. Slopes as steep as 2:1 will be allowed for other areas, with proper stabilization.

ADVANTAGES / BENEFITS:

- Moderate removal rate of urban pollutants.
- High community acceptance.
- Useful for water quality treatment and flood control.

DISADVANTAGES / LIMITATIONS:

- Potential for thermal impacts/downstream warming.
- Dam height restrictions for high relief areas.
- Pond drainage can be problematic for low relief terrain.

MAINTENANCE REQUIREMENTS:

- Remove debris from inlet and outlet structures.
- Maintain side slopes and outlet structure.
- Remove invasive vegetation.
- Monitor sediment accumulation and remove periodically.

OTHER CONSIDERATIONS:

- Outlet clogging
- Safety bench
- Landscaping

STORMWATER MANAGEMENT SUITABILITY

- Water Quality
- Channel/Flood Protection
- Overbank Flood Protection
- Extreme Flood Protection

Accepts runoff from SPAP land uses: Yes

FEASIBILITY CONSIDERATIONS

- M-H Land Requirement
- L Capital Cost
- L Maintenance Burden

Residential/Subdivision Use: Yes

High Density/Ultra-Urban: No

Drainage Area: 75 acres max.

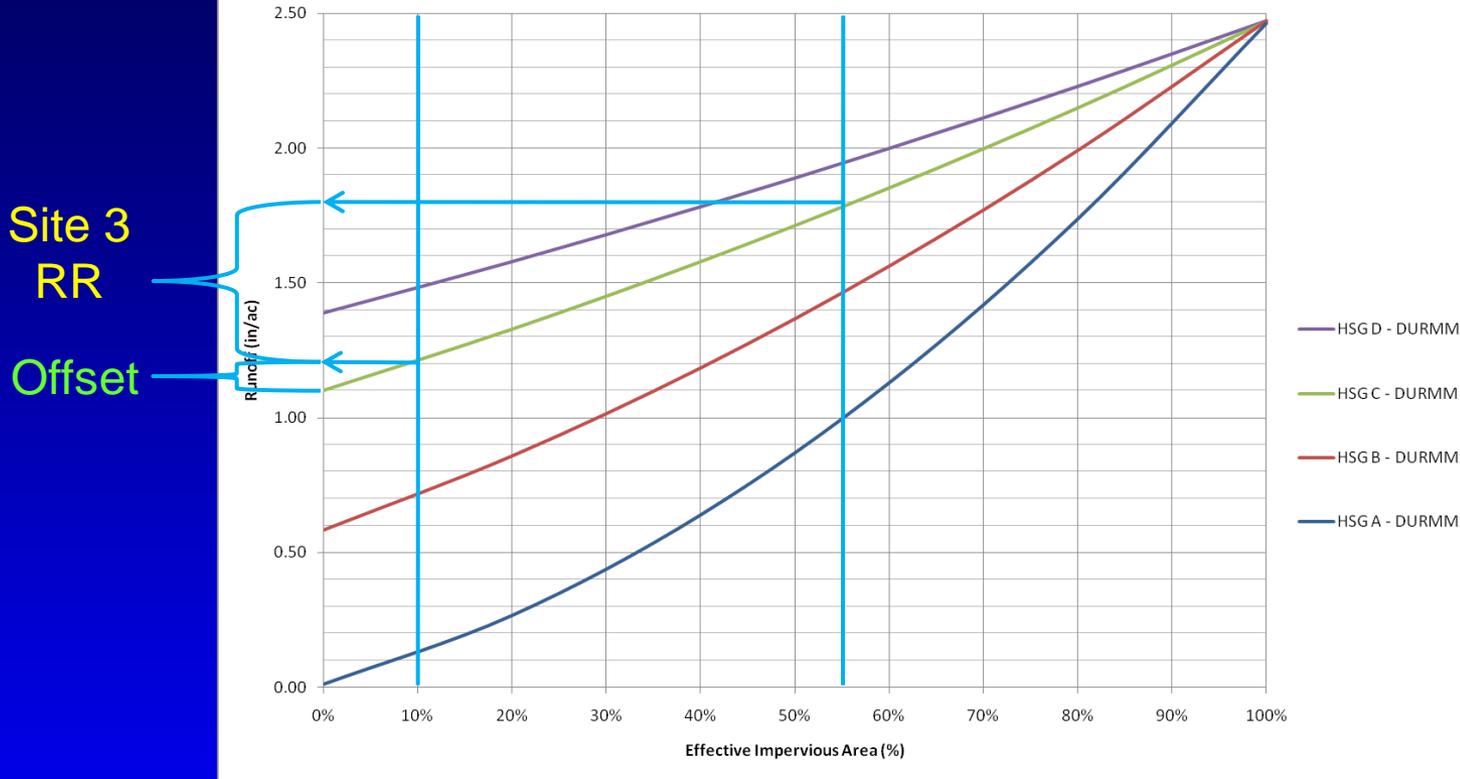
POLLUTANT REMOVAL

- M Total Suspended Solids
- L Nutrients: Total Phosphorus / Total Nitrogen
- L Metals: Cadmium, Copper, Lead, and Zinc
- No Data Pathogens: Coliform, Streptococci, E.Coli

L=Low M=Moderate H=High

- **TN: 20%**
- TP: 20%
- TSS: 60%

Runoff vs. Effective Impervious Area
(Resource Protection Event)



Original Offset Fee = $\$23/\text{cf} \times 363 \text{ cf}/\text{ac} = \$8,349/\text{ac}$

Offset Fee w/Dry Extended Detention Treatment BMP

Removal Efficiency for TN = 20%

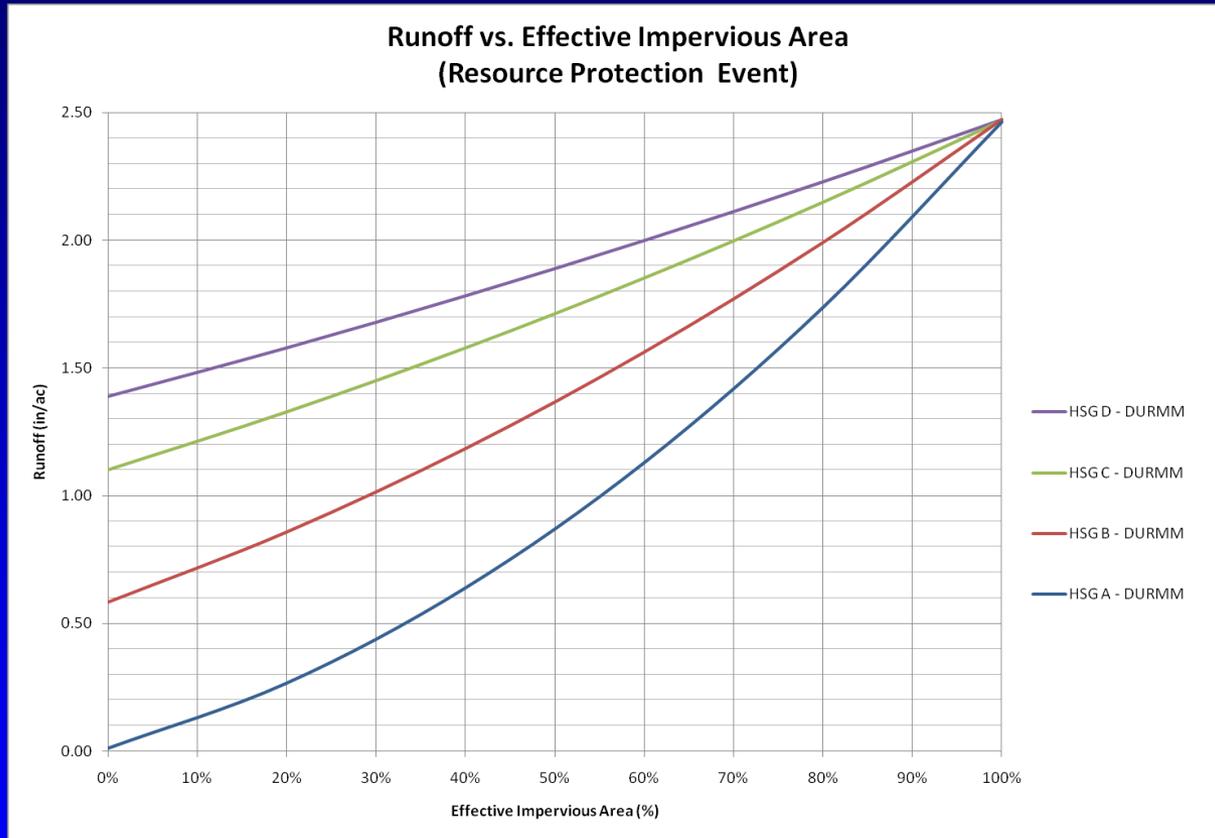
Fee Adjustment = $0.20 \times \$8,349 = \$1,670$

Adjusted Fee = $\$8,349 - \$1,670 = \$6,679/\text{ac}$

5.6 Redevelopment Criteria

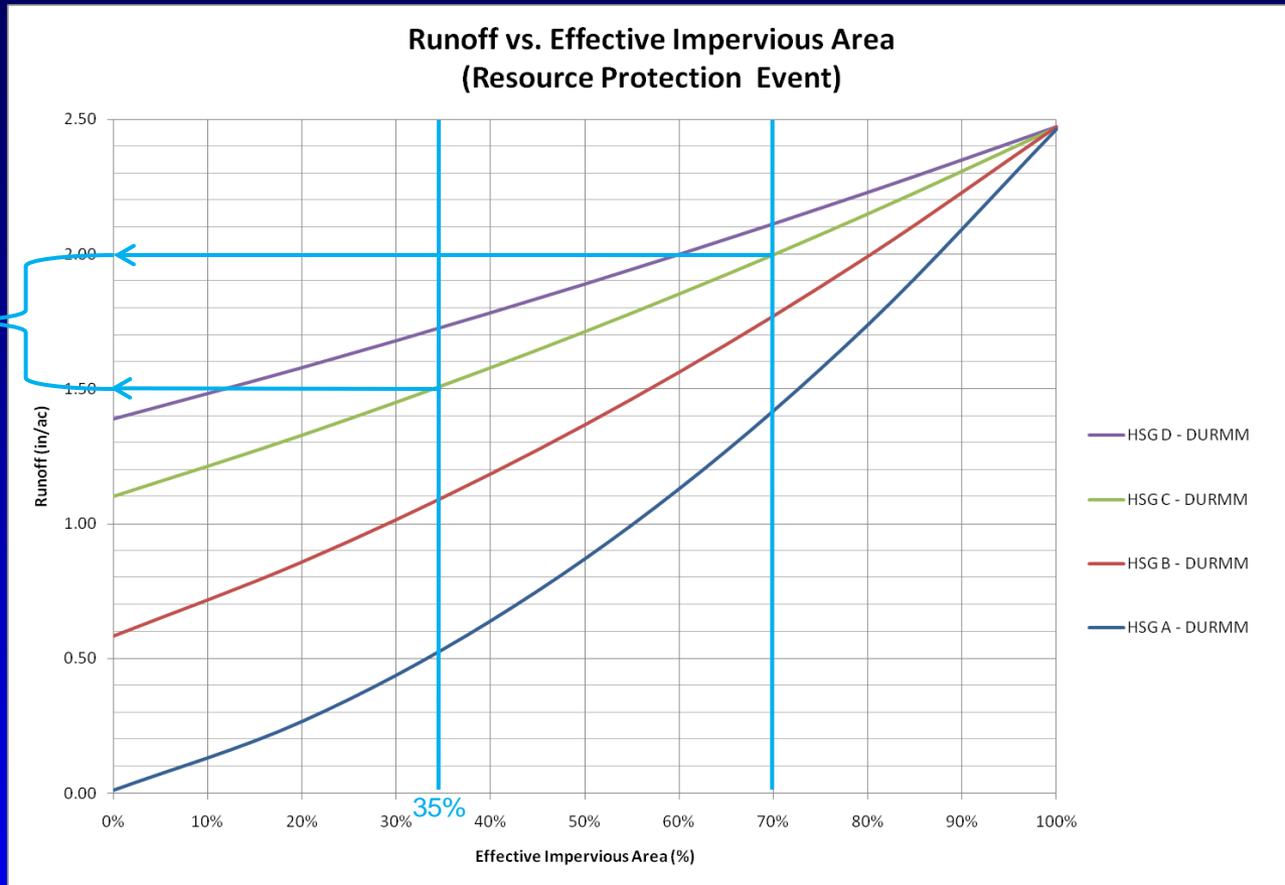
- *Section 5.6.3.1: Runoff from redeveloped areas within the project limit of disturbance that were wooded or meadow in the existing condition shall be reduced to an equivalent wooded condition using runoff reduction practices.*
- *Section 5.6.3.2: All remaining redeveloped areas within the project limit of disturbance shall employ runoff reduction practices to achieve a 50% reduction in the effective imperviousness based on the **existing condition**.*

Proposed Minimum RR for Redevelopment



50% Reduction in **Existing Effective** Imperviousness

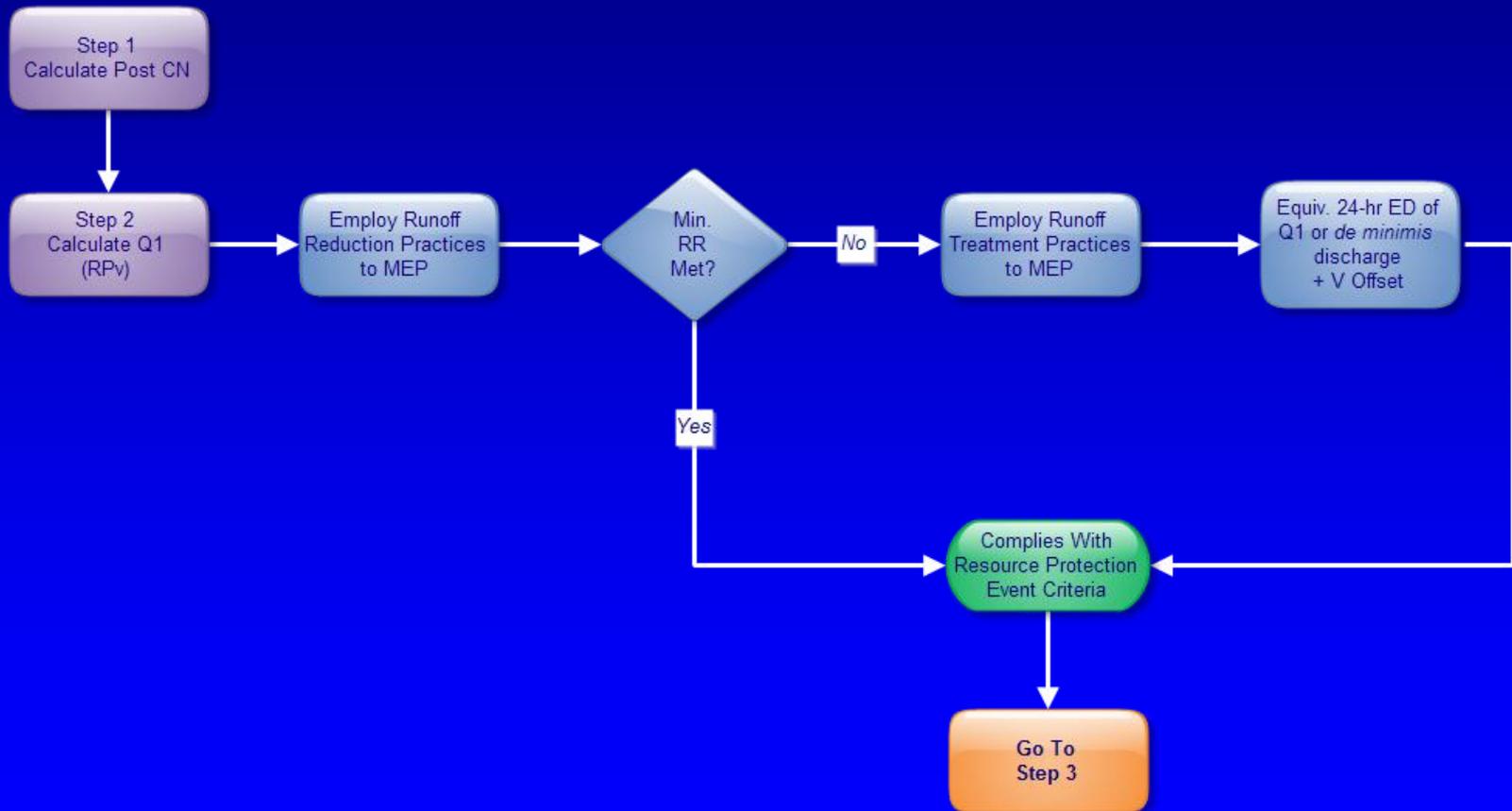
Redev.
Site RR



Redevelopment Site: 70% Ex. & Prop. Imperviousness, HSG C Soil
Runoff = 2.0”
Redeveloped Effective Imperviousness = 0.5 (70%) = 35%
Maximum Allowable Runoff for Compliance = 1.50”

(NOTE: New impervious areas that exceed the existing condition are treated the same as new development.)

5.2 Resource Protection Event Criteria



*Proposed Revisions to Delaware
Sediment & Stormwater Regulations*

Technical Document Update

DURMM v.2

DURMM_v2 2011-04-25_AM_bu.xls [Compatibility Mode] - Microsoft Excel

File Home Insert Page Layout Formulas Data Review View Developer

Clipboard Font Alignment Number Styles Cells Editing

Normal_WAT... Normal_WAT... Normal_WAT...
Normal Bad Good

			Curve Numbers for Hydrologic Soil Type							
Cover Type	Treatment	Hydrologic Condition	A		B		C		D	
			Acre	RCN	Acre	RCN	Acre	RCN	Acre	RCN
FULLY DEVELOPED URBAN AREAS (Veg Established)										
Open space (Lawns, parks etc.)										
	Poor condition; grass cover < 50%		68	79	86	89				
	Fair condition; grass cover 50% to 75 %		49	69	79	84				
	Good condition; grass cover > 75%		39	61	74	80				
Impervious Areas										
	Paved parking lots, roofs, driveways		98	98	98	98				
	Streets and roads									
	Paved: curbs and storm sewers		98	98	98	98				
	Paved: open ditches (w/right-of-way)		83	89	92	93				
	Gravel (w/ right-of-way)		76	85	89	91				
	Dirt (w/ right-of-way)		72	82	87	89				
Urban Districts										
	Commercial & business	Avg % impervious: 85	89	92	94	95				
	Industrial	72	81	88	91	93				
Residential districts by average lot size										
	1/8 acre (town houses)	Avg % impervious: 65	77	85	90	92				
	1/4 acre	38	61	75	83	87				
	1/3 acre	30	57	72	81	86				
	1/2 acre	25	54	70	80	85				
	1 acre	20	51	68	79	84				
	2 acre	12	46	65	77	82				
	User defined urban		**	**	**	**				
DEVELOPING URBAN AREA (No Vegetation)										
	Newly graded area (previous only)		77	86	91	94				
			0	0	0	0				
Total Acres			0							
Weighted Runoff Curve Number (RCN)			0							

Legend:
 User input
 Pre-set or output
 Result

Workflow →

CLEAR TABLE

C.A. RCN LOD OLOD RPV TMDL CV Fv DURMM Report Data & Documentation

DURMM v.2

Distributed vs. Sequential BMPs

DURMM_v2 2011-08-22 Example Calcs.xls [Compatibility Mode] - Microsoft Excel

PROJECT: Example
 DRAINAGE SUBAREA ID: 1A
 LOCATION (County): Sussex

RESOURCE PROTECTION EVENT (RPV) WORKSHEET

RESET

	BMP 1	BMP 2	BMP 3	BMP 4	BMP 5
	Impervious disconnection	Bioswale	Ephemeral wetland	--	--
Type	Type	Type	Type	Type	Type
Data	Data	Data	Data	Data	Data
1.1 Total contributing area to BMP (ac)	15.00	15.00	15.00	15.00	15.00
1.2 Reserved					
1.3 Initial RCN	67.58				
1.4 RPV for Contributing Area (in.)	0.82				
1.5 Req'd RPV Reduction for Contributing Area (in.)	0.36				
1.6 Req'd RPV Reduction for Contributing Area (%)	43%				
1.7 RPV allowable discharge rate (cfs)	0.23				
Step 2 - Adjust for Retention Reduction					
2.1 Storage volume (cu. ft.)					1000
2.2 Retention reduction allowance (%)	0%	0%	0%	N/A	N/A
2.3 Retention reduction volume (ac-ft)	0.00	0.00	0.00	N/A	N/A
2.4 Retention reduction volume (in.)	0.00	0.00	0.00	N/A	N/A
2.5 Runoff volume after retention reduction (in.)	0.82	0.74	0.56	N/A	N/A
2.6 Adjusted CN*	67.58	65.58	60.40	N/A	N/A
Step 3 - Adjust for Annual Runoff Reduction					
3.1 Annual CN (ACN)	67.58	65.58	60.40	N/A	N/A
3.2 Annual runoff (in.)	10.15	9.14	6.85	N/A	N/A
3.3 Proportion A/B soils in BMP footprint (%)			0%		
3.4 Annual runoff reduction allowance (%)	10%	25%	10%	N/A	N/A
3.5 Annual runoff after reduction (in.)	9.14	6.85	6.17	N/A	N/A
3.6 Adjusted ACN	65.58	60.40	58.61	N/A	N/A
3.7 Annual Runoff Reduction Allowance for RPV (in.)	0.08	0.26	0.32	N/A	N/A
Step 4 - Calculate RPV with BMP Reductions					
4.1 RPV runoff volume after all reductions (in.)	0.74	0.56	0.50	N/A	N/A
4.2 Total RPV runoff reduction (in.)	0.08	0.26	0.32	N/A	N/A
4.3 Total RPV runoff reduction (%)	9%	32%	39%	N/A	N/A
4.4 Adjusted CN after all reductions	65.58	60.40	58.61	N/A	N/A
4.5 Equivalent TR-55 RCN for H&H modeling	74.44	70.17	68.67	N/A	N/A
4.6 Required reduction met?	NO	NO	NO	N/A	N/A
4.7 If required reduction met, reduction credit (in.)	N/A	N/A	N/A	N/A	N/A
Step 5 - Determine Runoff Reduction Offset					
5.1 Runoff Reduction Shortfall (in.)	0.28	0.09	0.04	N/A	N/A
5.2 Runoff Reduction Shortfall (cu.ft./ac)	1,007	341	131	N/A	N/A
5.3 Total Offset Volume (cu.ft.)	15,107	5,111	1,967	N/A	N/A

DURMM v.2

Redevelopment – LOD Sheet

DURMM_v2 2011-08-25 HSG Beta.xls [Compatibility Mode] - Microsoft Excel

	B	C	D	E	F	G	H	I	J	K	L	P	Q	R	S	T	U	V	
1	PROJECT: 0																		
2	DRAINAGE SUBAREA ID: 0																		
3	LOCATION (County): 0																		
4	UNIT HYDROGRAPH: 0																		
5	LIMIT OF DISTURBANCE (LOD) WORKSHEET																		
6	Step 1 - Subarea LOD Data																		
7	1.1 HSG Area Within LOD (ac)	HSG A	HSG B	HSG C	HSG D	RESET													
9	1.3 Pre-Developed Impervious Within LOD (ac)																		
11	1.4.b Post-Developed Imperviousness Within LOD, Option #2 (%)	0%	0%	0%	0%														
13	Step 2 - Subarea LOD Runoff Calculations																		
14	2.1 RCN per HSG	0.00	0.00	0.00	0.00														
15	2.2 Rpv per HSG (in.)	0.00	0.00	0.00	0.00														
16	2.3 Target Runoff per HSG (in.)	0.00	0.00	0.00	0.00														
17	2.4 Cv Weighted Unit Discharge per HSG (cfs/ac)	0.00	0.00	0.00	0.00														
18	2.5 Fv Weighted Unit Discharge per HSG (cfs/ac)	0.00	0.00	0.00	0.00														
20	2.6 Subarea LOD (ac)	0.00																	
21	2.7 Subarea Weighted RCN	#DIV/0!																	
22	2.8 Subarea Weighted Rpv (in.)	#DIV/0!																	
23	2.9 Subarea Weighted Target Runoff (in.)	#DIV/0!																	
25	Step 3 - Upstream LOD Areas (from previous DURMM Report as applicable)																		
26	3.1 Upstream Sub-Area ID	Area 1	Area 2	Area 3	Area 4														
27	3.2 Upstream LOD Area (ac)																		
28	3.3 Target Runoff for Upstream Area (in.)																		
29	3.4 Adjusted CN after all reductions																		
30	3.5 Adjusted Rpv (in.)																		
31	3.6 Adjusted Cv (in.)																		
32	3.7 Adjusted Fv (in.)																		
34	Step 4 - Rpv Calculations for Combined LOD																		
35	4.1 Combined LOD (ac)	0.00																	
36	4.2 Weighted RCN	#DIV/0!																	
37	4.3 Weighted Rpv (in.)	#DIV/0!																	
38	4.4 Weighted Target Runoff (in.)	#DIV/0!																	
39	4.5 Estimated Annual Runoff (in.)	#DIV/0!																	
40	4.6 Req'd Runoff Reduction within LOD (in.)	#DIV/0!																	
41	4.7 Req'd Runoff Reduction within LOD (%)	#DIV/0!																	
43	Step 5 - Cv Unit Discharge																		
44	5. LOD Allowable Unit Discharge (cfs/ac)	#DIV/0!																	
46	Step 6 - Fv Unit Discharge																		
47	6. LOD Allowable Unit Discharge (cfs/ac)	#DIV/0!																	

DURMM v.2 & DNREC Nutrient Protocol

The screenshot shows a spreadsheet with a table titled "Curve Numbers for Hydrologic Soil Type". The table has columns for "Soil Type" (A, B, C, D) and "Curve Number" (A, B, C, D). The rows list various land use categories and their associated curve numbers.

Cover Type	Treatment	Hydrologic Condition	Curve Numbers for Hydrologic Soil Type			
			A	B	C	D
65	Impervious Areas		98	98	98	98
66	Paved parking lots, walk, driveways		98	98	98	98
67	Streets and roads		98	98	98	98
68	Paved courts and storm sewers		98	98	98	98
69	Paved open ditches (ungrazed)		79	89	89	89
70	Gravel (or right-of-way)		79	89	89	89
71	Dirt (or right-of-way)		73	83	83	83
72	Urban Districts		69	62	64	63
73	Commercial & business	Avg % Impervious	61	62	64	63
74	Industrial		61	62	64	63
75	Residential districts by average lot size	Avg % Impervious	77	69	80	82
76	10 acre (low houses)	36	61	79	83	87
77	1/2 acre	50	67	72	81	88
78	1/2 acre	75	64	79	80	83
79	1 acre	80	61	69	79	84
80	2 acre	12	46	65	77	83
81	DEVELOPING URBAN AREA (No Vegetation)		77	69	80	82
82	Newly graded area (permeous only)		77	69	80	82
83	USER DEFINED					
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The screenshot shows a spreadsheet titled "Nutrient Loading Assessment Protocol Work Sheet (Version May, 2011)". It includes a map of Chesapeake Bay with various drainage basins highlighted in different colors. A table lists the drainage basins and their corresponding watershed IDs.

Drainage Basins	Chesapeake Bay Drainage
1	Stomachs Creek
2	Shadybrook Creek
3	Blackwater Creek
4	Red Clay Creek
5	White Clay Creek
6	Choptank River
7	Delaware River
8	Jerry Creek
9	Choptank River
10	Choptank River
11	C & D Canal East
12	Appomattox River
13	Blackbird Creek
14	Delaware Bay
15	Stomachs River
16	Choptank River
17	Little Creek (Little Creek)
18	James River
19	Muskeget River
20	Roanoke River
21	Center Creek
22	Roanoke River
23	Leaves/Roadside Canal
24	Roadside Canal
25	Indian River
26	Indian River
27	Indian River
28	Indian River
29	Indian River
30	Indian River
31	Indian River
32	Indian River
33	Indian River
34	Indian River
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47	Indian River
48	Indian River
49	Indian River
50	Indian River

= "DURMMocol"

DURMM v.2 & DNREC Nutrient Protocol

NutrientProto050911-DRAFT.xls [Compatibility Mode] - Microsoft Excel

Update "Result Summaries" Tab

Parcel Information

The proposed Development is	Lands of Shipbuilder's LLC		
The Tax Parcel Number is	2-30.26.115.115.04 & 122.00		
The proposed development is in	Kent County		
The proposed development is in the TMDL area established for the	CHESAPEAKE BAY DRAINAGE		
The total acreage of the Parcel is	100.00		
The total acreage of the proposed development in the Parcel is	100.00		

Overall Loading Summary

	TN	TP	SED
Pre-development Nutrient Load (lb/day)	8.10	0.54	977.33
Target Nutrient Load (lb/day)	4.64	0.41	781.95
Post-development Nutrient Load - Without BMPs (lb/day)	4.70	0.45	652.46
Post-development Nutrient Load (lb/day)	5.15	0.49	652.46

With your proposed BMPs, your proposed development meets required TMDL reduction for Sediment but not for TN and TP!

Loading Summary

	TN	TP	SED
Stormwater BMP and/or Urban Buffer Reductions (lb/day) Enter values from the DURMM model	0.00	0.00	0.00
TMDL Required Nutrient Reductions (%)	25	24	20
Target Nutrient Load (lb/day)	4.64	0.41	781.95
Post development nutrient Load due to landuse conversion only (lb/day)	5.15	0.49	652.46
Wastewater Treatment Load (lb/day)	0.10	0.00	
Post development Nutrient Load + Wastewater Treatment Load (lb/day)	5.25	0.49	652.46
Post development Nutrient Load + Wastewater Treatment - [Stormwater BMPs and/or Urban Buffer Reductions] (lb/day)	5.25	0.49	652.46
Post development Nutrient Load + Wastewater Treatment - [Stormwater BMPs and/or Urban Buffers + Offsite Septic Elimination if occurring] (lb/day)	5.15	0.49	652.46
Post development Nutrient Load + Wastewater Treatment - [Stormwater BMPs and/or Urban Buffers + Offsite Septic Elimination + Septic Tank Pumpout if occurring] (lb/day)	5.15	0.49	652.46
Post-development Nutrient Load + Wastewater Treatment - [Stormwater BMPs and/or Urban Buffers + Offsite Septic Elimination + Septic Tank Pumpout if occurring + Agricultural BMPs] (lb/day)	5.15	0.49	652.46

Note: All loads are delivered loads

Loading Summary By Landuse with BMPs applied

LANDUSE	CURRENT (lb/day)			PROPOSED (lb/day)		
	TN	TP	SED	TN	TP	SED
Animal feeding operations	0.00	0.00	0.00	0.00	0.00	0.00
Residential	0.00	0.00	0.00	0.00	0.00	0.00

**Post-Dev
LOD Only!**

H&H Analyses for Preliminary S&S Plans

DRAFT – 2011-08-16

Procedure for Conducting Hydrologic & Hydraulic Analyses For Preliminary Sediment & Stormwater Plans

Background

The Hydrologic & Hydraulic (H&H) Analysis couples field collected data with desktop watershed modeling methodology to provide a tool for stormwater management agencies to help determine the most appropriate method to manage stormwater runoff from developing sites based on "No Adverse Impact" principles. The general approach is one of increasing level of detail and analysis depending on the complexity of the watershed. It is NOT intended to be a substitute for detailed Watershed Master Plans that have been endorsed by the Department. When such detailed plans are available, the peak discharge and/or volume management requirements from the Watershed Master Plan shall take precedence over the requirements of the Level 2 Analysis. Additionally, the methodologies used for this analysis are not considered to be precise enough to be applied at the site level.

Procedure

1. Applicability

- 1.1. The H&H analysis will be required for all projects using the performance-based option and/or where a sump condition exists.

2. Methodology

2.1. Level 1 Analysis

- 2.1.1. The Level 1 Analysis combines field reconnaissance data with hydrologic modeling of the upstream watershed and site using latest soils, LULC, and terrain data. Hydrographs are then compared to check for coincidental peaking effects.
- 2.1.2. Limit of study shall be the most-downstream junction of the site and the upstream contributing area.
- 2.1.3. To comply at this level of analysis, hydrologic modeling must indicate no adverse impact due to coincidental peaking effects. For purposes of this policy, "no adverse impact" shall mean that the developed site hydrograph peak is less than, and the inflection point occurs before, the peak of the upstream hydrograph OR that it can be demonstrated that on-site detention would exacerbate downstream impacts. If compliance cannot be demonstrated, proceed to Level 2.

2.2. Level 2 Analysis

- 2.2.1. The Level 2 Analysis combines field measurement data with hydraulic modeling of structures, channels, etc. using an expanded hydrologic model.

H&H Analyses for Preliminary S&S Plans

Background

The Hydrologic & Hydraulic (H&H) Analysis couples field collected data with desktop watershed modeling methodology to provide a tool for stormwater management agencies to help determine the most appropriate method to manage stormwater runoff from developing sites based on “No Adverse Impact” principles. The general approach is one of increasing level of detail and analysis depending on the complexity of the watershed. It is NOT intended to be a substitute for detailed Watershed Master Plans that have been endorsed by the Department. When such detailed plans are available, the peak discharge and/or volume management requirements from the Watershed Master Plan shall take precedence over the requirements of the Level 2 Analysis. Additionally, the methodologies used for this analysis are not considered to be precise enough to be applied at the site level.

H&H Requirements

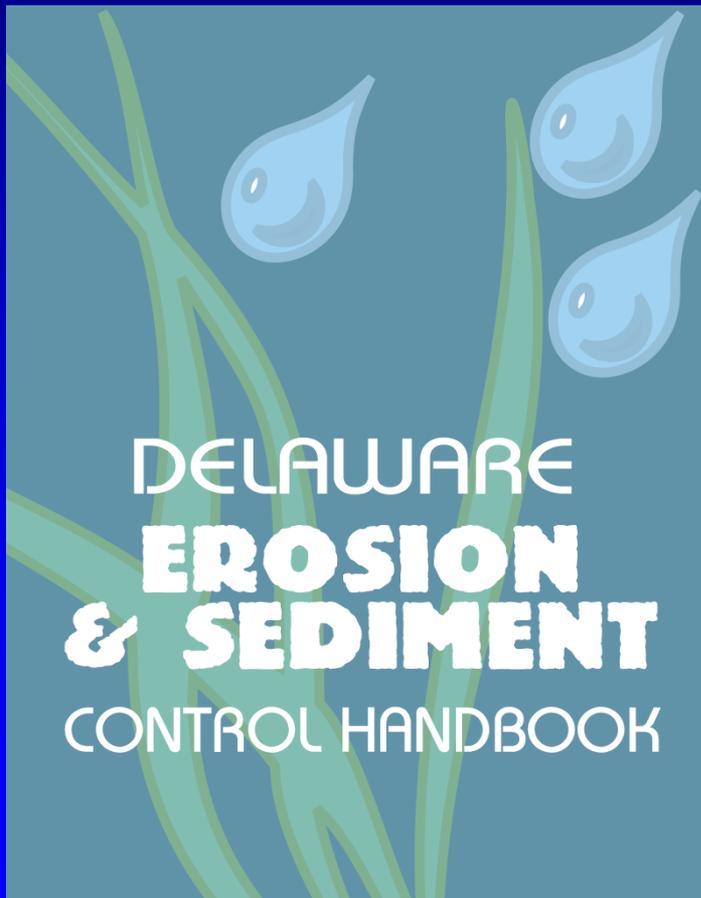
Level 2 Analysis

2.2.3. To comply at this level of analysis, the applicant must show that:

2.2.3.1. The runoff volume based on the NRCS Runoff Curve Number (RCN) and the rate of runoff based on the calculated peak discharge for the post-developed condition does not exceed that for the pre-developed condition, or;

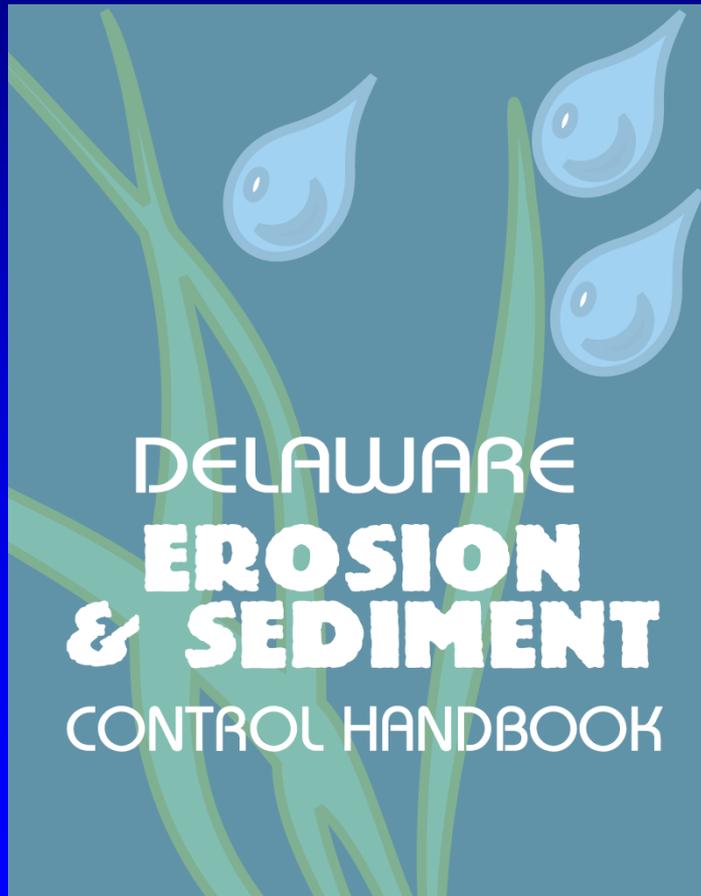
2.2.3.2. Steady flow hydraulic modeling must indicate no adverse impacts to headwater, water surface elevations and/or areas of inundation at designated POAs. For purposes of this policy, “no adverse impact” shall mean less than 0.05’ increase in the calculated water surface elevations in channels and/or in headwater at hydraulic structures for all points of analysis. In addition, the area of inundation shall not encroach upon buildings or similar structures previously not impacted. If ~~significant impacts occur~~ the “no adverse impact” condition can not be met, a remedy must be provided in accordance with Section 3 of this policy. If impacts are uncertain, proceed to Level 3.

Delaware ESC Handbook Updates



- Revisions
 - Culvert Inlet Protection (delete Silt Fence option)
 - Inlet Protection (add Type-3 for curb opening)
 - Tree Protection (expand to Sensitive Area Protection)

Delaware ESC Handbook Updates



- New
 - Compost
 - Log
 - Blanket
 - Trap
 - Concrete Washout
 - Flocculants
 - Small Batch Plants
 - Stockpiles

GTBMP Stds & Specs

“New & Improved”

- Infiltration Practices
- Bioretention
- Permeable Pavement
- Vegetated Roofs
- Rainwater Harvesting
- Dry Swales
- Impervious Disconnection
- Grass Channel
- Sheet Flow to Filter Strip or Open Space
- Detention Practices
- Filtering Practices
- Wetlands
- Stormwater Ponds
- Soil Amendments
- Proprietary Practices

GTBMP Stds & Specs

“New & Improved”

Stormwater BMP Design Criteria

Infiltration

1.0 Stormwater Infiltration

Definition: Practices that capture and temporarily store the design storm volume before allowing it to infiltrate into the soil over a two day period. Design variants include:

- I-1 infiltration trench
- I-2 infiltration basin

Infiltration practices use temporary surface or underground storage to allow incoming stormwater runoff to exfiltrate into underlying soils. Runoff first passes through multiple pretreatment mechanisms to trap sediment and organic matter before it reaches the practice. As the stormwater penetrates the underlying soil, chemical and physical adsorption processes remove pollutants. Infiltration practices are suitable for use in residential and other urban areas where *measured* soil permeability rates exceed 1/2 inch per hour. To prevent possible groundwater contamination, infiltration should not be utilized at sites designated as stormwater hotspots. Extraordinary care shall be taken to assure that long-term infiltration rates are achieved through the use of performance bonds, post construction inspection and long-term maintenance.

1.1 Infiltration Stormwater Credit Calculations

Infiltration practices receive 100% retention volume credit (R_v) for the amount of storage volume (S_v) provided by the practice (Table 1.1). No additional pollutant removal credit is awarded.

Calculation Method	Retention of RP _v Event
Retention Credit (R_v)	100% of S_v
Annual Pollutant Removal Credits	
TN	$100\% * R_v / RP_v$
TP	$100\% * R_v / RP_v$
TSS	$100\% * R_v / RP_v$

The practice must be sized using the guidance detailed in *Section 1.5. Infiltration Design Criteria*

GTBMP Stds & Specs

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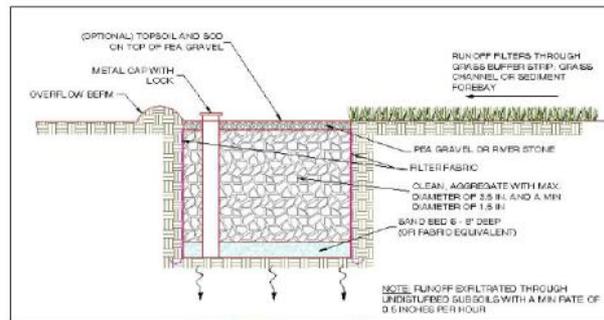


Figure 1.1. Example of an Infiltration Trench.

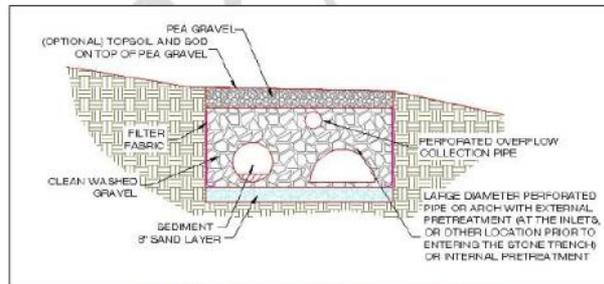


Figure 1.2. Infiltration Section with Supplemental Pipe Storage.

GTBMP Stds & Specs

“New & Improved”

1.2 Design Criteria and Materials Specifications

Table 1.2 summarizes design criteria for infiltration practices, and Table 1.3 summarizes the materials specifications for these practices. For more detail, consult Sections 1.3 through 1.7. Sections 1.8 describes practice construction and maintenance criteria.

	Basins	Trenches
Feasibility (Section 1.3)	<ul style="list-style-type: none"> Minimum soil infiltration of 1"/hr Restrictions for treating hotspots, high loads, or dry weather flows 2 foot separation from groundwater Setbacks from wells, buildings and utilities Typically used to treat small (<2 acre) CDA with high impervious cover 	
Conveyance (Section 1.4)	<ul style="list-style-type: none"> Can be designed off-line or on-line Must safely convey the 10-year storm event. 	
Pretreatment (Section 1.5)	<ul style="list-style-type: none"> 25% to 50% of Sv in pretreatment, depending on soil infiltration rate. All runoff must be treated. Several pretreatment options may be used. 	
Sizing (Maximum Depth) (Section 1.6)	$d_{max} = \frac{1}{2} i \times t_d$	$d_{max} = \frac{1}{2} i \times t_d / \eta_r$
	Maximum depth limits as well, based on practice size and CDA (See Table 1.4)	
Sizing (Surface Area) (Section 1.6)	$SA = Sv / (d + \frac{1}{2} i \times t_f)$	$SA = Sv / (\eta_r \times d + \frac{1}{2} i \times t_f)$
Variables:	<ul style="list-style-type: none"> t_d = maximum drawn down time (normally 1.5 to 2 days) (day) i = field-verified infiltration rate for the native soils (ft./day) η_r = available porosity of the stone reservoir (assume 0.35) Sv = Design Storage volume (Sv) treated by the practice d = Infiltration depth (ft.) t_f = Time to fill the infiltration facility (days – typically 2 hours, or 0.083 days) 	
Geometry (Section 1.6)	<ul style="list-style-type: none"> Flat trench or basin bottom. 4:1 or flatter internal side slopes for basins 2' or lower ponding depth for basins 	<ul style="list-style-type: none"> Wider than they are deep to avoid injection well status.
Landscaping (Section 1.7)	Maintain vegetation in the buffers and practice drainage area to minimize erosion and debris.	

GTBMP Stds & Specs

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Table 1.3. Infiltration Material Specifications

Material	Specification	Notes
Surface Layer (optional)	Topsoil and grass layer	
Surface Stone	Install a 3-inch layer of river stone or pea gravel.	This provides an attractive surface cover that can suppress weed growth.
Stone Layer	Clean, aggregate with a maximum diameter of 3.5 inches and a minimum diameter of 1.5 inches.	
Observation Well	Install a vertical 6-inch Schedule 40 PVC perforated pipe, with a lockable cap and anchor plate.	Install one per 50 feet of length of infiltration practice.
Overflow collection pipe (optional)	Use 4-inch or 6-inch rigid schedule 40 PVC pipe, with 3/8" perforations at 6 inches on center, with each perforated underdrain installed at a slope of 1% for the length of the infiltration practice.	
Trench Bottom	Install a 6 to 8 inch sand layer (e.g., ASTM C 33, 0.02-0.04 inch)	
Filter Fabric (sides only)	Use non-woven polypropylene geotextile with a flow rate of > 110 gallons/min./sq. ft. (e.g., Geotex 351 or equivalent).	
Buffer Vegetation	Keep adjacent vegetation from forming an overhead canopy above infiltration practices, in order to keep leaf litter, fruits and other vegetative material from clogging the stone.	

1.3 Infiltration Feasibility Criteria

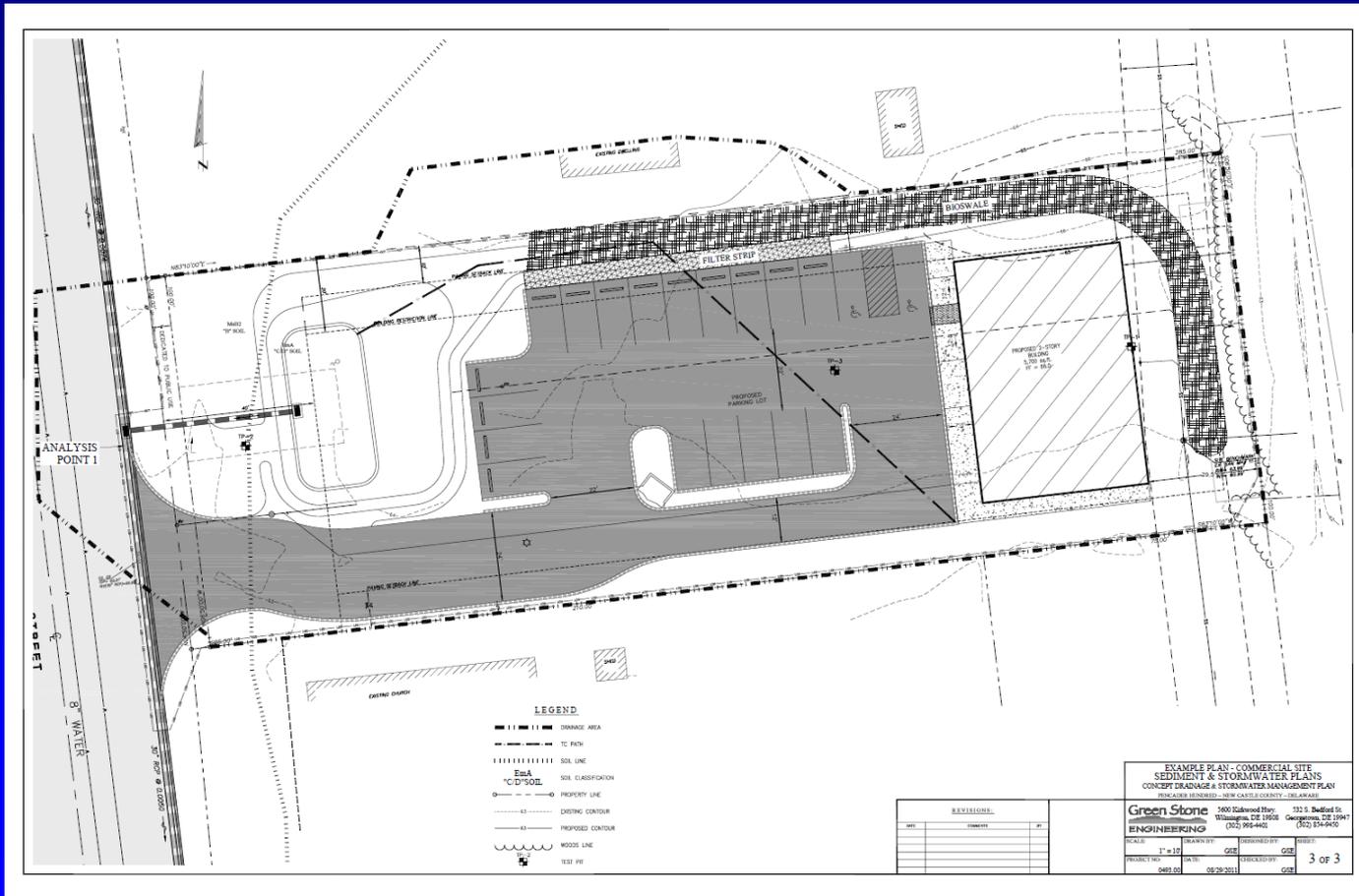
Infiltration practices have very high storage and retention capabilities when sited and designed appropriately. Designers should evaluate the range of soil properties during initial site layout and seek to configure the site to conserve and protect the soils with the greatest recharge and infiltration rates. In particular, areas of Hydrologic Soil Group A or B soils shown on NRCS soil surveys should be considered as primary locations for infiltration practices. Additional information about soil and infiltration are described in more detail later in this section. During initial design phases, designers should carefully identify and evaluate constraints on infiltration, as follows:

Underground Injection Control for Class V Wells. In order for an infiltration practice to avoid classification as a Class V well which is subject regulation under the Federal Underground Injection Control (UIC) program, the practice must generally be wider than the practice is deep. If an infiltration practice is “deeper than its widest surface dimension,” or if it includes an underground distribution system then it will likely be considered a Class V injection well. Class V injection wells are subject to permit approval by EPA. For more information on Class V injection wells and stormwater management, designers should consult: http://www.epa.gov/mpdes/pubs/memo_gi_classvwells.pdf

Contributing Drainage Area. The maximum contributing drainage area (CDA) to an individual infiltration practice should be less than 2 acres and as close to 100% impervious as possible. The design, pretreatment and maintenance requirements will differ depending on the size of the infiltration

Example Preliminary Sediment & Stormwater Plans

Example Preliminary S&S Plan Commercial Site



DURMM v2
 Example Plan - Commercial Site
 Stormwater Management Report

For

DNREC
 Div. of Watershed Stewardship
 Sediment and Stormwater Program

Prepared by:

Green Stone
 ENGINEERING
 1600 Kalkreuth Highway
 Wilmington, DE 19806

August 2011

Proposed Revisions to Delaware Sediment & Stormwater Regulations

What's Next?

Timeline Update

- Sept – Dec 2011: Technical Document
- Nov 2011: Technical Subcommittee
- Jan 2012: RAC Meeting
- Feb 2012: Public Hearing
- May 2012: Promulgation
- Aug 2012: Effective Date

Summer 2012

- Promulgation in May 2012
- Training throughout summer
 - DURMMv2
 - Standards & Specifications
- Effective in August 2012

Grandfathering of Projects In the Review Process

- Will apply at August 2012 effective date
- Plans under review: 1 year to gain approval under previous regs
- Must meet grandfathering criteria of each delegated agency
 - Policy will be posted on website

Plans Approved Prior to Effective Date

- Valid for 3 years
- May be extended if construction commences
- Only phases with approved construction plans may be extended

Questions???

RegRevisions - Windows Internet Explorer

http://www.swc.delaware.gov/Drainage/Pages/RegRevisions.aspx

Delaware.gov | Text Only

Governor | General Assembly | Courts | Elected Officials | State Agencies



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DNREC : Division of Soil & Water Conservation : Drainage and Stormwater Section



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- DNREC Public Notices

Services

- Conservation Districts
- Contractor Services
- Debris Pit Program
- Environmental Navigator
- Environmental Navigator (Revised - Beta version)
- Loans/Grants/Cost-Share
- Macroalgae Harvesting
- Permits/Licenses/Approval
- Restoration

Information

- Alphabetical Listing of Information
- Delaware Estuarine Research Reserve
- Regs/Laws
- Request for Qualifications
- SWC Publications & Newsletters

Revisions To The Delaware Sediment And Stormwater Regulations

Revisions to the Delaware Sediment and Stormwater Regulations are currently under way. To assist with this effort, the Delaware Sediment and Stormwater Program has contracted with a consultant team consisting of the Center for Watershed Protection, Johnson, Mirmiran & Thompson, and the Horsely Witten Group.

Regulatory Advisory Committee (RAC)
The Regulatory Advisory Committee (RAC) was formed to help guide the revisions to the Delaware Sediment and Stormwater regulations.

Upcoming RAC Meetings: TBD
[Find details on all DNREC meeting locations and times](#)

RAC Meeting Summaries To Date

- May 27, 2010 [Agenda, Meeting Notes, Presentation](#)
- Feb. 25, 2010 [Agenda, Meeting Notes, Presentation](#)
- Feb. 9, 2009 [Agenda, Meeting Notes, Presentation](#)
- March 27, 2008 [Agenda, Meeting Notes](#)
- Jan. 22, 2008 [Agenda, Meeting Notes](#)
- Oct. 16, 2007 [Agenda, Meeting Notes](#)

RAC Subcommittees & Meeting Summaries

Documents

- [DRAFT Technical Document](#) - Sept. 2010
- [DURMMv2](#) - July 2010
- [Second Draft](#) - May 2010
- [First draft comment responses](#) - May 2010
- [September 2009 RAC Update Memo](#)
- [Stormwater Assessment Report](#) (Final)
- [First working draft of Sediment and Stormwater Regulations](#) Feb. '09
- [See comments on first working draft regs under RAC Subcommittees](#)
- [Update memo to RAC](#) Aug. '08
- [Subcommittee Outline Comments](#) March '08
- [Gov. Minner's Task Force on Surface Water Management](#) April 2005

Sign Up to Receive Updates
If you wish to receive regulatory revision updates and notices of public meetings related to revisions to the regulations, please send an e-mail containing your contact information to Elaine.Webb@state.de.us.

start | 0-Data | Technical Subcommittee | Microsoft PowerPoint... | RegRevisions - Windo... | 11:21 AM

Discussion

